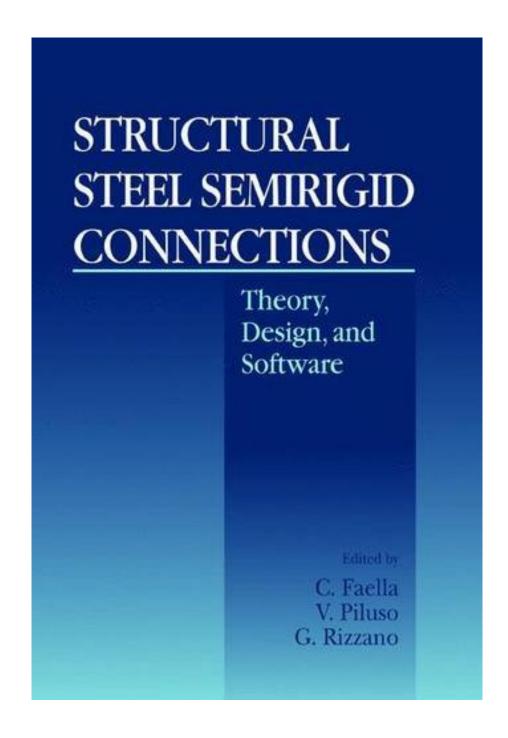


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Although the semirigidity concept was introduced many years ago, steel structures are usually designed by assuming that beam-to-column joints are either pinned or rigid. Theses assumptions allow a great simplification in structural analysis and design-but they neglect the true behavior of joints.

The economic and structural benefits of semirigid joints are well known and much has been written about their use in braced frames. However, they are seldom used by designers, because most semirigid connections have highly nonlinear behavior, so that the analysis and design of frames using them is difficult. In fact, the design problem becomes more difficult as soon as the true rotational behavior of beam-to-column joints is accounted for-the design problem requires many attempts to achieve a safe and economical solution.

Structural Steel Semirigid Connections provides a comprehensive source of information on the design of semirigid frames, up to the complete detailing of beam-to-column connections, and focuses on the prediction of the moment-rotation curve of connections. This is the first work that contains procedures for predicting the connection plastic rotation supply-necessary for performing the local ductility control in nonlinear static and dynamic analyses. Extensive numerical examples clarify the practical application of the theoretical background.

This exhaustive reference and the awareness it provides of the influence of joint rotational behavior on the elastic and inelastic responses of structures will greatly benefit researchers, professionals, and specification writing bodies devoted to structural steel.

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For the longest time, joints were treated as pinned or rigid. Neither extreme is, strictly-speaking, usually applicable to reality, and recent trends in engineering have factored the semi-rigidity of connections. Steel semirigid connections can have most of the properties of rigid connections, yet affect a savings of weight.

The term "semirigid connections" actually spans a large range of stiffness. The least-stiff steel-to-steel

semirigid connection I found in this book (p. 266) had a kNm/rad value of 1,410. This corresponds to Z=1.0X10(E-6). Other semirigid joints are 10-60 times stiffer than this.

In this book, there is a balance between theory and practical application. Joint strength, ductility, fatigue behavior, etc., are considered. A software CD-rom is included.

This work is limited to semirigid joints in steel connections. Timber mortise-and-tenon joints also exhibit semirigid properties, albeit at 10-100 times less rigidity than the least-stiff semirigid steel connections. It is hoped that a future edition of this book will include discussion of semirigid joints in timber engineering.

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It's amazing the way Mr Faella describes the steel connections on this book, showing the real structural mechanism in the connections. I guess it more suitable for researchers and advanced users. For most cases the use of the standards rigid or pinned(Shear or moment) connections will drive to acceptables solutions.

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